

## Research Article

### INVESTIGATION OF EFFECTIVE FACTORS ON PLUNGE POOL SCOUR DEPTH

<sup>1</sup>Mitra Kalantari and <sup>2</sup>Jalal Bazargan

<sup>1</sup>Department of Civil Engineering, University of Zanjan, Zanjan, I. R. of Iran

#### ARTICLE INFO

##### Article History:

Received 28<sup>th</sup>, September 2015  
Received in revised form  
06<sup>th</sup>, October 2015  
Accepted 15<sup>th</sup>, November 2015  
Published online 30<sup>th</sup>, December 2015

##### Keywords:

Dimensional Analysis,  
Empirical Relations,  
Flip-Bucket,  
Free Falling Jet,  
Plunge Pool,  
Scour Hole.

#### ABSTRACT

High energy dissipation of the free falling jet from the flip-bucket in the plunge pools will cause downstream river erosion. While simple and economic implementation of such structures has attracted the attention of most designers. Scour due to falling jet incidence with the river bed will threaten the stability of the dam and related structures, and hence determination of the plunge pool dimensions was an important design consideration. The purpose of this study is to develop empirical exponential relations using the principles of statistics and dimensionless relations by applying dimensional analysis in order to estimate the scour depth due to falling jets considering the effective parameters such as discharge per unit width, the difference between water levels in the reservoir and tailwater (jet fall height), tailwater depth, the mean diameter of bed particles and flip-bucket jet angle. By comparing the results of the suggested relations in this study with other investigator results it can be indicated that the developed relations have the maximum correlation coefficient and minimum computational error in an appropriate standard deviation range. Also, the accuracy and dispersion of the 139 collected data series that were used for the proposed relations was higher than the previous studies.

Copyright © 2015 Mitra Kalantari and Jalal Bazargan. This is an open access article distributed under the Creative Commons Attribution License, which permits unrestricted use, distribution, and reproduction in any medium, provided the original work is properly cited.

#### INTRODUCTION

Scour phenomenon is in fact the displacement of particles from their original location to another one. A particle starts moving when the applied forces by the flow, i.e. the shear and lift forces that separate particles from the bed overcome the weight of the particle. Scour occurs when flow conditions and erodible bed particle and is one of the main causes of damage and failure of hydraulic structures. Despite the long history of scour phenomenon in hydraulics science, since the safe and economic design of hydraulic structures that are located in the flow path requires a proper estimation of the maximum scour depth, presenting a relation that considers all the conditions and existing complexities has been still of particular interest to the Hydraulics and River engineering researchers. Hydraulic structures as flow barriers change the flow pattern nearby, and cause local scour. Investigation of the scour phenomenon becomes important when scour depth is significant so that it reaches the river structure foundation and threatens the stability of the structures or destroys them. One of the most common methods of dissipating the energy of the flow passing a chute is to use plunge pools and falling jets.

Despite the method being economic for the energy dissipation, the falling jet incidence to the downstream river bed will cause scour hole. When the flow is discharged to the pools, the energy is diffused and reduced. Bed scour induced by the jet incidence to the erodible bed with respect to the jet type can be classified into the following groups (Guide No.549 2011).

- Scour due to vertical jets
- Scour due to a horizontal jets
- Scour due to free falling jets. each of the above groups can be classified into one of the following groups:
  - Submerged or free jet;
  - High or short falling jet;
  - Aerated or non-aerated jet. In general, scour phenomenon includes the two following stages:

In the first stage, by displacement of the alluvial materials, jet hydrodynamic forces will break bed materials. In the second stage, separated parts are moved from the original location by the falling water jet and the expansion of the scour hole occurs.

#### Damages can be prevented and/or minimized by either

- Prevention of scour;
- Limitation of the scour location and extent. Due to economic considerations, usually the second option is used, with the

\*Corresponding author: Mitra Kalantari,  
M. Sc. graduated, Department of Civil Engineering, University of Zanjan, Zanjan, I. R. of Iran.

goal to control and limit the scoring under the framework of the hydraulic structure.

LITERATURE REVIEW

Numerous empirical relations for prediction of the maximum scour depth downstream of a dam with a jet spillway have been obtained by performing experiments on physical models.

Detail design of the plunge pools based on USBR recommendations are given in Figure 1.

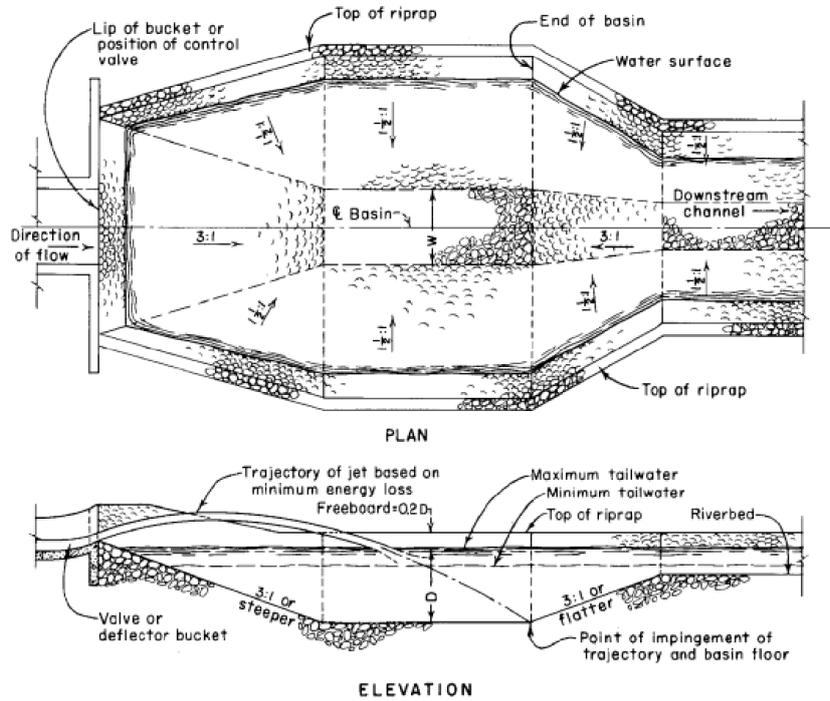


Fig. 1. Plunge pool energy dissipater (Design of small dams 1987)

Table 1. Empirical relations presented by other researchers (Azamatullah et al. 2005; D’Agostino & Ferro 2004; Ghodsian et al.2012; Kuroiwa & Minaya 2005; Mason & Arumugam 1985; Veronese 1937; Yen 1987)

Researcher	Year				
Veronese-A	1937	$y_s = 0.202 q^{0.54} H^{0.225} / d_m^{0.42}$	Martins-B	1975	$y_s = 1.5q^{0.6} H^{0.1}$
Jaeger	1939	$y_s = 0.6q^{0.5} H^{0.25} \left(\frac{y_t}{d_{50}}\right)^{0.333}$	Taraimovich	1978	$y_s = 0.663q^{0.67} H^{0.25}$
Chee-Padiyar	1969	$y_s = 2.126 q^{0.67} H^{0.18} / d_m^{0.063}$	Sofrelec	1980	$y_s = 2.3q^{0.6} H^{0.1}$
Chee-Kung	1974	$y_s = 1.663q^{0.6} H^{0.2} / d_m^{0.1}$	Mason	1985	$y_s = 3.27 \frac{q^{0.6} H^{0.05} y_t^{0.15}}{g^{0.3} d_{50}^{0.1}}$
Mason-Arumugam	1985	$y_s = \left(6.42 - (3.1 * H^{0.1})\right) \frac{q^{(0.6 - \frac{H}{300})} H^{0.15 + \frac{H}{200}} y_t^{0.15}}{g^{0.3} d_{50}^{0.1}}$			
Yen	1987	$y_s = \left(\frac{q^2}{g}\right)^{0.34} \left(6.42 - (3.1H^{0.1})\right) \frac{H}{600} \left(\frac{gH^3}{q^2}\right)^{0.2 + \frac{H}{200}} \left(\frac{H}{d_{50}}\right)^{0.1} \left(\frac{y_t}{H}\right)^{0.15}$			
Azmathullah	2005	$\frac{y_s}{y_t} = 6.914 \left(\frac{q}{\sqrt{y_t^3 g}}\right)^{0.694} \left(\frac{H}{y_t}\right)^{0.0815} \left(\frac{R}{y_t}\right)^{-0.233} \left(\frac{d_{50}}{y_t}\right)^{0.196} (\phi)^{0.196}$			
Kuroiwa-Minaya	2005	$\frac{h_s}{y} = 3.188 \left(\frac{q}{z((G-1)gd_{50})^{0.5}}\right)^{0.3875} \left(\frac{V}{\sqrt{gy}}\right)^{1.377} \left(\frac{z}{H}\right)^{0.6254} \left(\frac{d_{85}}{d_{50}}\right)^{0.1185} \left(\frac{H}{y_t / \sin\theta}\right)^{0.196}$			
Ghodsian	2012	$\frac{\phi}{Y_t} = x_1 Fr_{d90}^{x2} \left(\frac{H}{R_H}\right)^{x3} \left(\frac{B}{b_1}\right)^{x4} \left(\frac{Y_t}{H}\right)^{x5}$			$Fr_{d90} = \frac{V}{\sqrt{gd_{90} (\rho_s / \rho - 1)}}$

Researchers have used different assumptions in developing these relations. Therefore, a wide range of scour depth results are predicted using these relations. Among the presented relations by the researchers for the scour hole depth prediction, those capable of predicting a close to reality depth, and also the relations having common parameters with the presented relations in this study are presented in Table 1. in which  $y_s$ : scour depth from tailwater level (m),  $q$ : discharge per unit width ( $m^2/s$ ),  $g$ : acceleration of gravity ( $m/s^2$ ),  $H$ : falling height (m),  $d_{50}$ : average diameter of bed particles (m),  $y_t$ : tailwater depth (m),  $\phi$ : flip-bucket angle (degree),  $\varphi$ : Dimensionless parameter,  $R_{Ht}$ : hydraulic radius,  $B$ : width (m),  $Fr$ : Froude number,  $V$ : flow velocity (m/s),  $\rho$ : fluid density ( $kg/m^3$ ),  $\rho_s$ : buoyant sediment density ( $kg/m^3$ ).

**APPLIED DATA**

The data used in this( as show in Table 2 and 3) study included 95 data sets collected by Azmatullah in order to predict the scour depth due to falling jet from the flip-bucket chute spillway (Azamatullah 2005), and 26 Laboratory data series presented by Kuroiwa-Minaya to investigate scour depth due to falling jet incidence with the non-cohesive bed of the plunge pool (Kuroiwa & Minaya 2005). Also, 18 data series collected from physical models of executive dams including Daryan, Azad and Chere dams having Flip-bucket dissipating energy system and falling jet incidence with the plunge pool by Iran Water Research Institute have been used (Final report of Dariyan dam 2013;Final report of Azad dam 2008; Final report of Chere dam2008).

**Table 2. data collected by researcher (Azmathullah et al.,2005; Kuroiwa Zevallos and Minaya Espinoza, 2005)**

	Azmatullah	Kuroiwa-Minaya
data	95	26
$q(m^2/s)$	0.0089-0.381	0.033-0.1
$H(m)$	0.2791-1.7962	0.401-0.974
$y_s(m)$	0.0512-0.55	0.11-0.599
$d_{50}(m)$	0.002-0.008	0.0016-0.049
$y_t(m)$	0.0286-0.265	0.05-0.5
$\phi(^{\circ})$	10-45	35

**Table 3.data collected from physical models of dams (Final report of Dariyan dam, 2013; Final report of Azad dam, 2008; Final report of Chere dam, 2008)**

	Dariyan dam	Azad dam	Chere dam
data	6	6	6
$q(m^2/s)$	20.24-143	17-76	20-114
$H(m)$	14.23-142.4	93-96	93.02-98.85
$y_s(m)$	14.2-38.58	19-51	16-46
$d_{50}(m)$	0.0075	0.008	0.008
$y_t(m)$	2.2-15.58	14-23	2.68-7.75
$\phi(^{\circ})$	39	46	46

**MATEIALS AND METHODS**

**Presenting exponential relations**

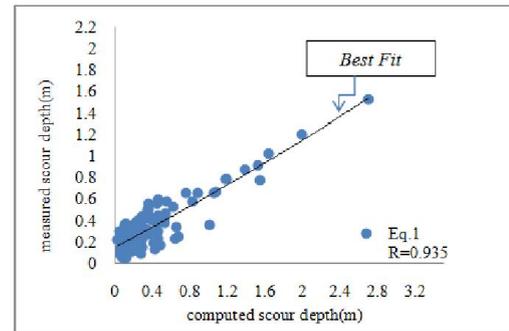
The proposed exponential relation using SPSS19 software was modified in order to achieve better results in the form of Equation 1. The evaluation results of this equation are given in Table 4.

$$y_s = 0.065136 q^{0.563} H^{0.001} d_{50}^{-0.01} y_t^{0.25} \phi^{0.974} \tag{1}$$

**Table 4. Evaluation results of exponential equation 1**

	<i>R</i>	<i>MAE</i>	<i>RMSE</i>	$\delta$
Eq.1	0.935	0.005	0.109	28.05

in which *R*: Correlation coefficient, *MAE*: Mean Average Error, *RMSE*: Root mean square error,  $\delta$ : average absolute deviation. In order to compare the data distribution, experimental and calculated scour depths are presented in Figure 2.



**Fig. 2. Comparison of the measured and computed scour depths**

**Evaluation of the presented exponential relation**

Comparison and validation of Equation 1 with the empirical relations having a more suitable data fitting with other presented relations, and also with the recommended relations by the USBR standard and No. 549 Issue is presented in Table 5.

**Table 5. Comparison of parameters from Equation 1 with other empirical equations**

	<i>R</i>	<i>MAE</i>	<i>RMSE</i>	$\delta$
Eq.1	0.935	0.005	0.109	28.05
Mason	0.85	-49.32	0.19	38.03
Mason-Arumugam	0.852	-51.99	0.2	40.54
chee-Padiyar	0.854	-78.09	0.294	62.97
Martins-B	0.867	-11.63	0.128	28.93
Sofrelec	0.867	-71.17	0.213	50.33
Taraimovich	0.874	-60.99	0.249	63.74
Yen	0.843	-57.35	0.191	40.5

Table 5 shows that the exponential Equation 1 produces the maximum correlation coefficient and the lowest computational error and lowest standard deviation. It therefore gives a better scour depth prediction. Figure 3 that imposes the results from the other empirical equations on Figure 1 further illustrates that Equation 1 predicts the depth of scour better especially for larger scour depths.

**Dimensionless relation**

**Dimensional Analysis**

Dimensionless relations obtained using empirical models can be generalized to different physical conditions. The purpose of dimensional analysis is to group effective variables of a physical phenomenon into dimensionless groups called  $\Pi$  terms.

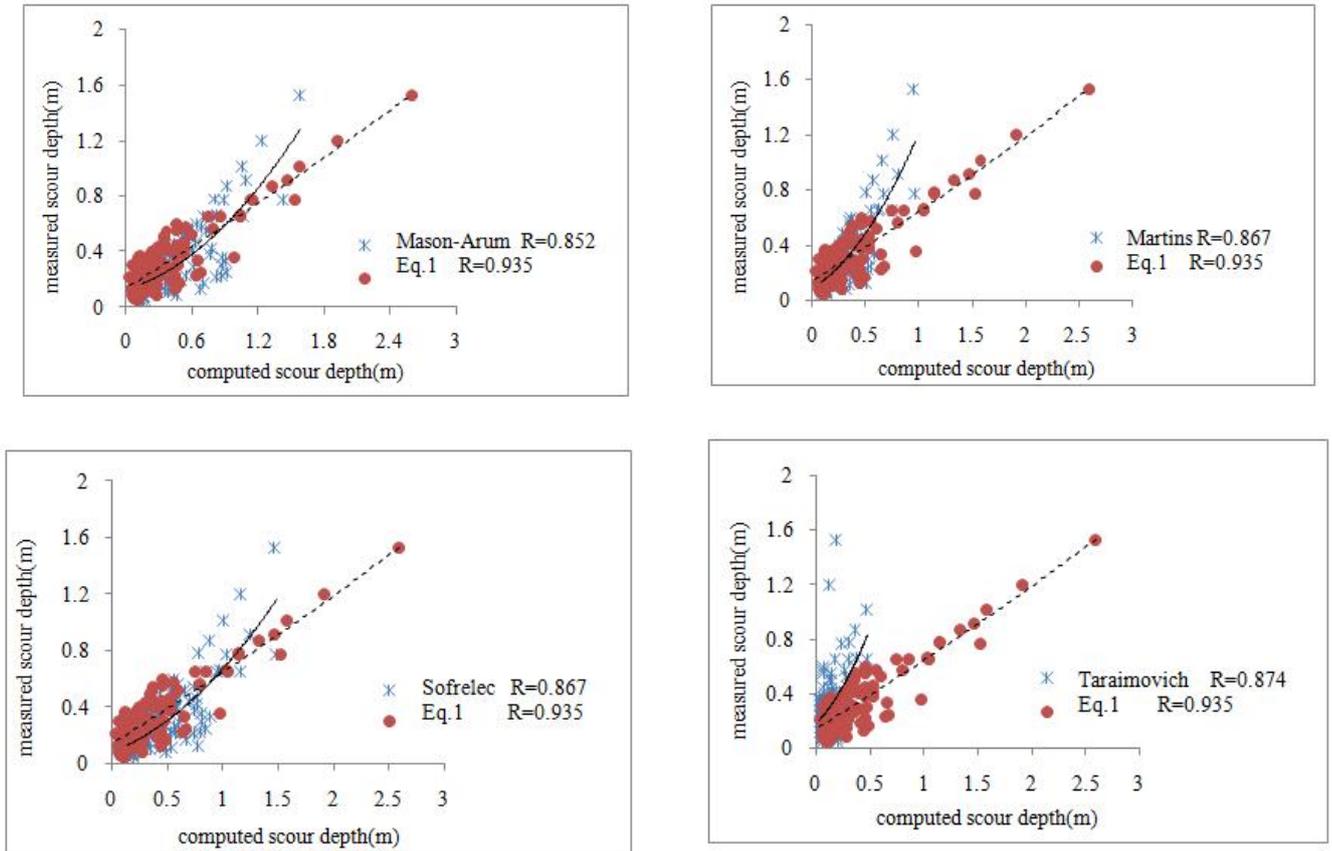


Fig. 3. Comparison of the exponential Equation 1 with empirical equations by other researchers

The advantage of using dimensional analysis is to reduce the number of variables. Among the dimensional analysis methods,  $\Pi$  Buckingham Theorem will be used. Variables associated with scour hole depth due to a falling jet (Fluid Mechanics-Book 2004).

$$f(y_s, d_{50}, q, H, y_t, g, \phi) = 0$$

$$\Pi_1 = f(\Pi_2, \Pi_3, \Pi_4, \Pi_5)$$

$$\Pi_1 = \frac{y_s}{y_t} \quad \Pi_2 = \frac{d_{50}}{y_t} \quad \Pi_3 = \frac{H}{y_t} \quad \Pi_4 = \frac{q}{\sqrt{y_t^3 g}} \quad \Pi_5 = \phi$$

$$\frac{y_s}{y_t} = f\left(\frac{d_{50}}{y_t}, \frac{H}{y_t}, \frac{q}{\sqrt{y_t^3 g}}, \phi\right) \tag{2}$$

By combining  $\Pi_2$  and  $\Pi_3$  dimensionless variables, a new dimensionless variable in the form of  $\Pi_6 = \frac{d_{50}}{H}$  is obtained.

Therefore, the rewritten equation takes the form of Equation 3:

$$\frac{y_s}{y_t} = f\left(\frac{d_{50}}{H}, \frac{q}{\sqrt{y_t^3 g}}, \phi\right)$$

$$\frac{y_s}{y_t} = \alpha \left(\frac{d_{50}}{H}\right)^a \left(\frac{q}{\sqrt{y_t^3 g}}\right)^b \phi^c \tag{3}$$

Finally, the proposed dimensionless relation is presented in the form of equation 4:

$$\frac{y_s}{y_t} = 0.0905 \left(\frac{d_{50}}{H}\right)^{-0.001} \left(\frac{q}{\sqrt{y_t^3 g}}\right)^{0.474} \phi^{0.999} \tag{4}$$

The evaluation of the parameters for dimensionless Equation 4 and deviation pattern are presented in Table 6 and Figure 4, respectively.

Table 6. Evaluation results of the dimensionless equation 4

	R	MAE	RMSE	$\delta$
Eq.4	0.936	0.005	0.107	28.55

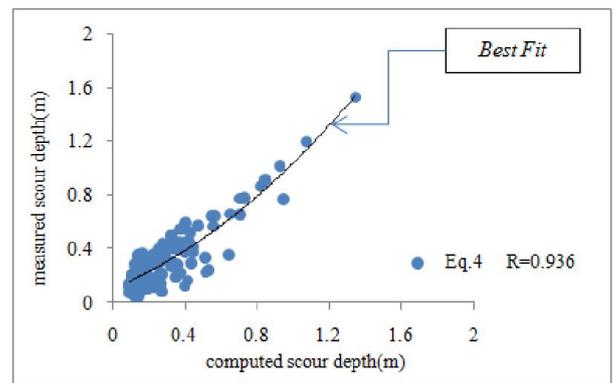
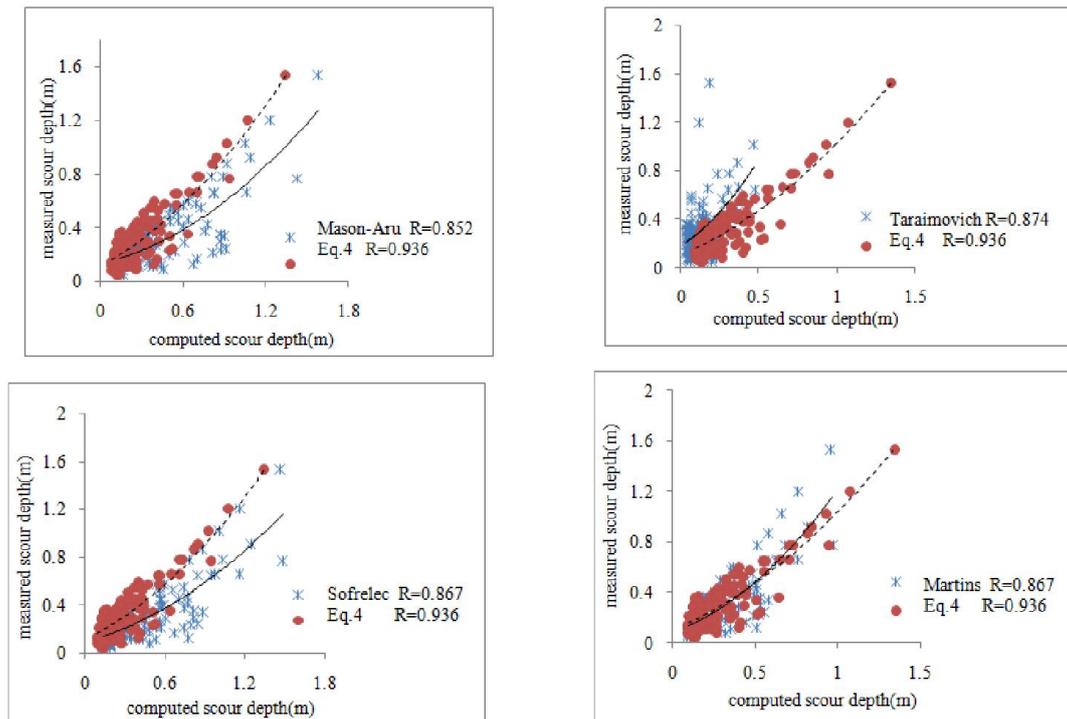


Fig. 4. Comparison of the measured and computed scour depths Evaluation of the proposed dimensionless relation

Comparison and validation results of dimensionless equation 4 with empirical relations by other researchers are given in Table 7. In order to compare the data fitting patterns and also the computational results of the proposed dimensionless relation with other relations Figure 5 is presented.

**Table 7. Comparison of the calculated results using the exponential relation with other empirical relations**

	<i>R</i>	<i>MAE</i>	<i>RMSE</i>	$\delta$
Eq.4	0.936	0.005	0.107	28.55
Mason	0.85	-49.32	0.19	38.03
Mason-Arumugam	0.852	-51.99		0.2 40.54
chee-Padiyar	0.854	-78.09	0.294	62.97
Martins-B	0.867	-11.63	0.128	28.93
Sofrelec	0.867	-71.17	0.213	50.33
Taraimovich	0.874	-60.99	0.249	63.74
Yen	0.843	-57.35	0.191	40.5



**Fig. 5. Comparison of the exponential dimensionless relation 4 with other researchers' relations**

### Capabilities of the proposed relations

To evaluate the capabilities of the exponential and dimensionless relation proposed in this study, using 139 collected data series, first  $r = \frac{y_{sm}}{y_s}$  ratio was calculated, where

$y_{sm}$  is the experimental scour depth and  $y_s$  is the measured scour depth by other researchers. The closer the ratio to 1, the more the accuracy of the proposed relation would be. Then, the percentage of the data with *r* values in the range of 0.5-2 was selected as the model rating (Hoffmans 1998). Ratings are given in Table 8.

**Table 8. Comparison of the proposed equations ratings with the empirical relations ratings**

evaluated relations	ratings (in percent)
Azmatullah	96.8
Eq.1	95
Eq.4	93.5
Martins-B	92.8
Mason	81.2
Mason-Arumugam	79.8
Yen	79.1
Sofrelec	77.6
Chee - Padiyar	74.1

Based on the obtained results, the rating of the presented exponential and dimensionless model of the present study was placed after the Azmatullah model rating indicating the high ability of the two models in predicting the scour depth. It is worth mentioning that Azmatullah has obtained his relation based on 95 collected data series and the model rating has been obtained using the same number of data series.

Therefore, a high rating for his relation is not unexpected. However, the presented relation by Azmatullah does not define the physical nature of the scour phenomenon properly, because the power of the average grading diameter of the particles parameter is positive and according to this relation, by increasing bed particle diameter scour depth is also increased, which is not a correct definition. The ratings related to the researchers models are shown in Figure 6.

Also, the rating of the proposed exponential and dimensionless model is shown in Figure 7. Based on the figures and considering the higher density of the points around the 45 degrees line for the proposed exponential and dimensionless relations in comparison to other researchers relations it can be concluded that both models have proper ability to predict the scour depth.

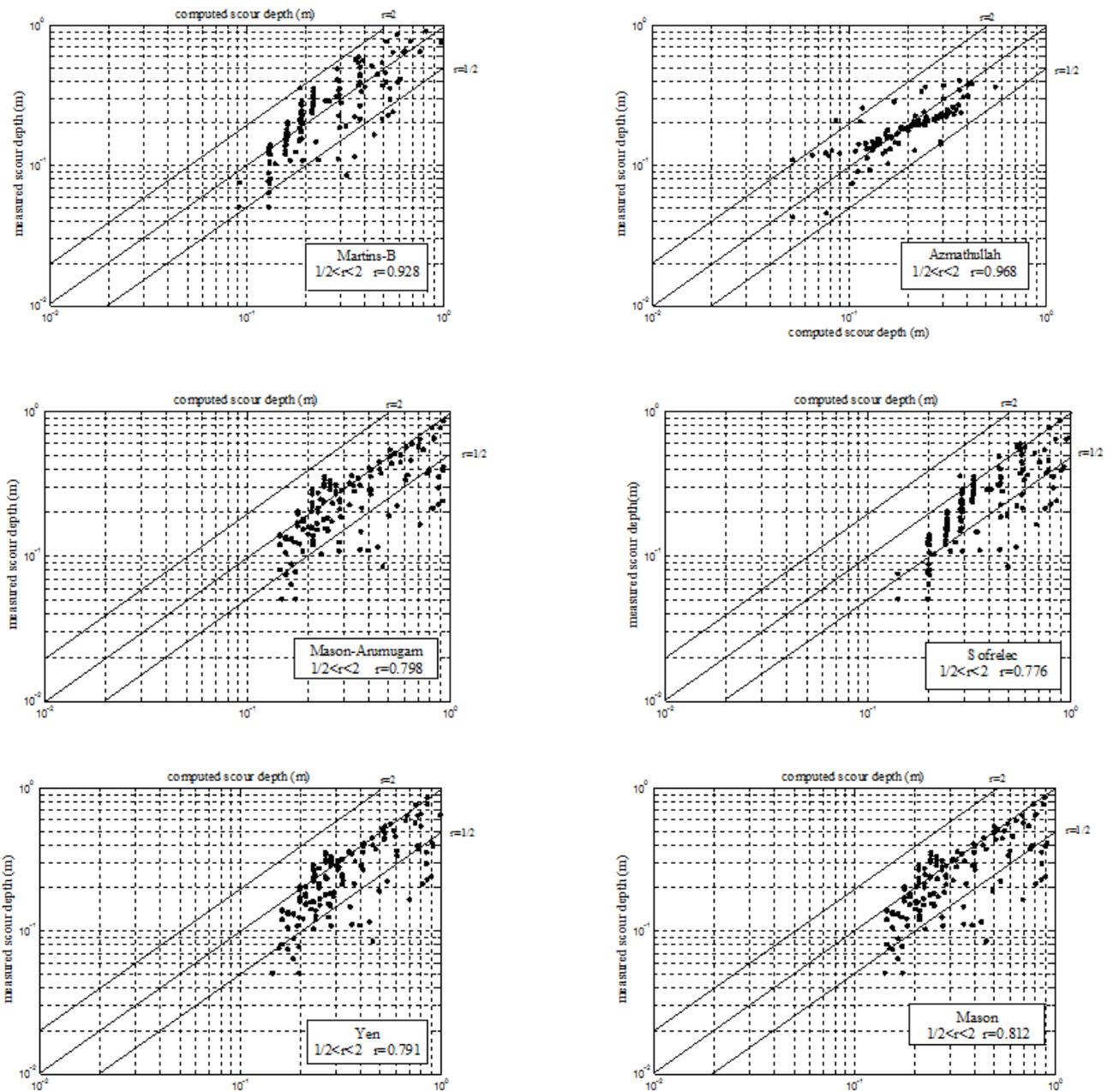


Fig. 6. Distribution of the data and ratings of the other investigators relations

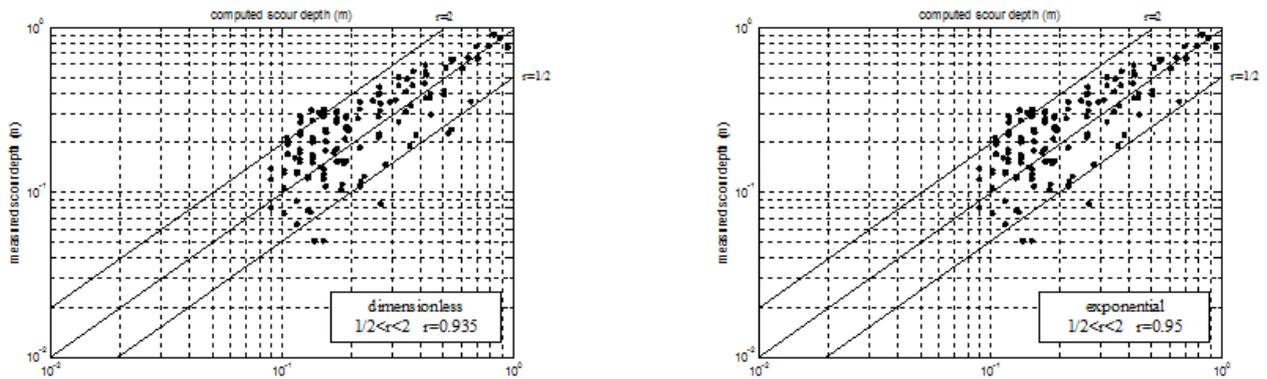


Fig. 7. Distribution of data and ratings of the proposed exponential and dimensionless models

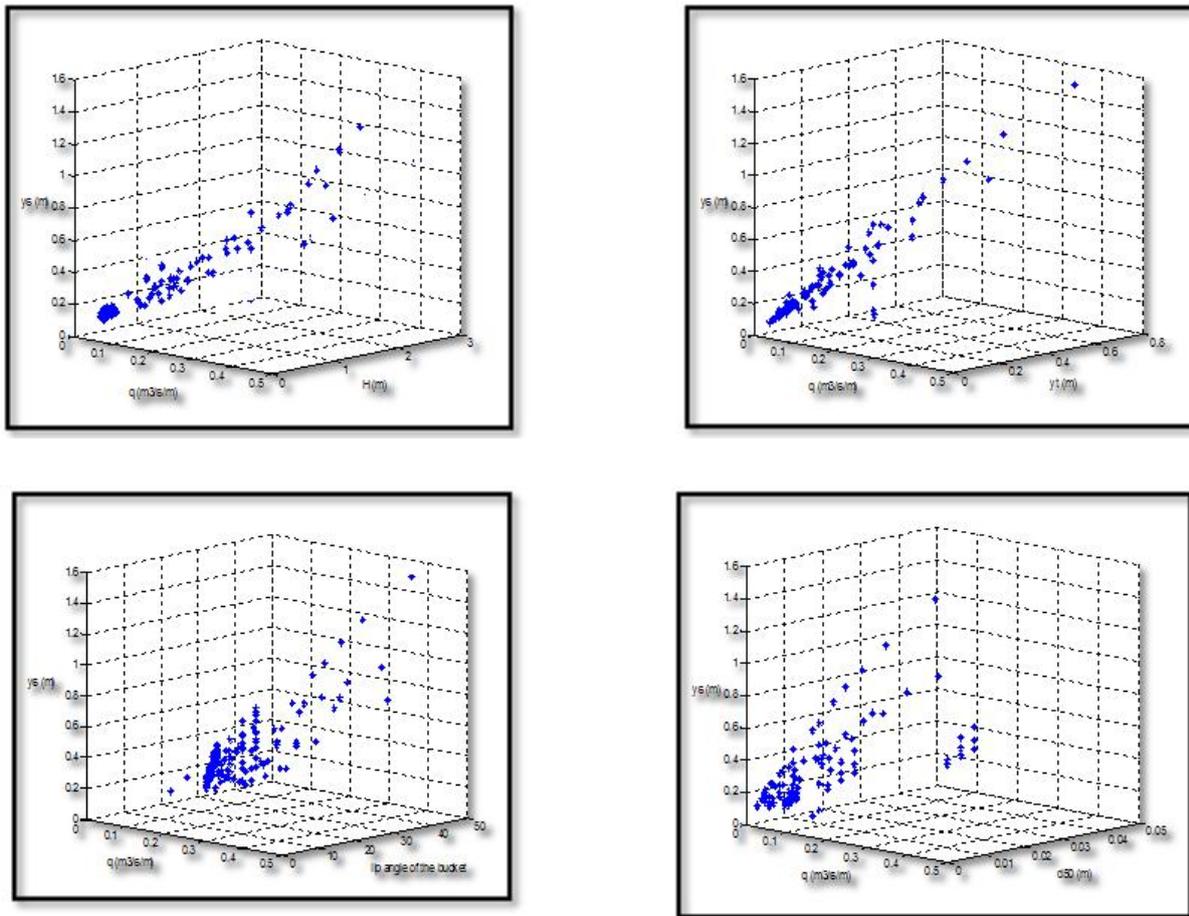


Fig. 8. Distribution of calculated scour depths versus other parameters

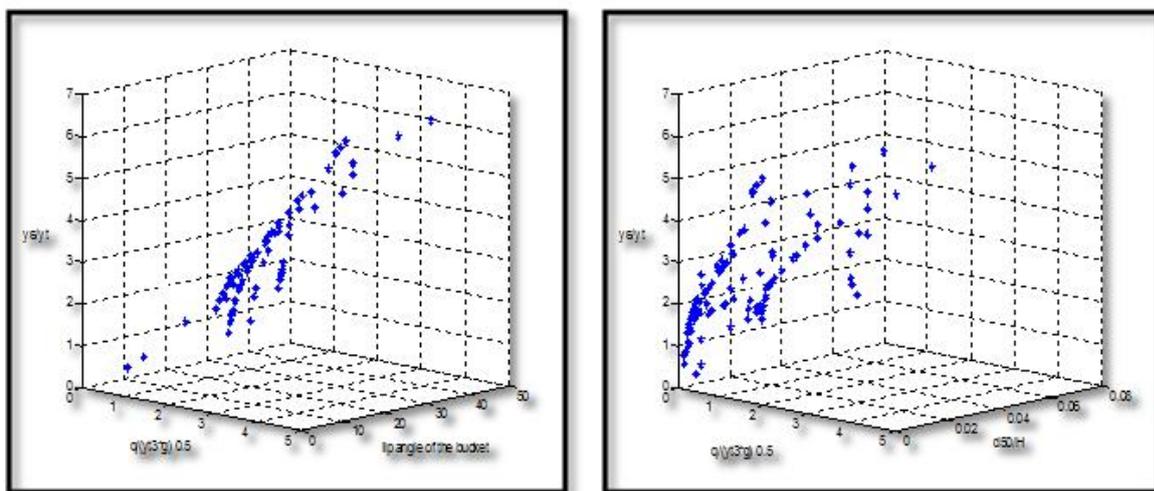


Fig. 9. Distribution pattern of scour depth versus other parameters

**DATA ANALYSIS**

In order to evaluate the effectiveness of each parameter on the proposed exponential and dimensionless relations and to present data distribution pattern on three-dimensional coordinate system, the correlation coefficient of each parameter is given along with the maximum calculated scour depth. The results indicated that the parameter including discharge per unit width is the most effective parameter and has the highest correlation coefficient among other parameters.

**Evaluation of the proposed exponential relation**

Computational results of the correlation coefficient between scour depth and other parameters are given in Table 9. To show the data distribution on three-dimensional coordinate system, by assuming constant values for calculated scour depth and discharge per unit width, the effect of other parameters is shown in Figure 8.

**Table 9. Correlation coefficient between calculated scour depth using the proposed exponential relation and other parameters**

	$y_s$	$q$	$y_t$	$d_{50}$	$H$	$\phi$
$R$		0.937	0.827	0.215	0.831	0.741

**Evaluation of the proposed dimensionless relation**

Scour depth results calculated by the proposed dimensionless relation and data distribution patterns are presented in Table 10 and Figure 9, respectively.

**Notation**

$y_s$	scour depth from tailwater level (m)	$r$	proposed model rating
$q$	discharge per unit width (m <sup>2</sup> /s)	$y_{sm}$	Experimental scour depth (m)
$g$	acceleration of gravity (m/s <sup>2</sup> )	$y_s$	Scour depth measured by researchers (m)
$H$	falling height (m)	$R$	Correlation coefficient
$d_{50}$	average diameter of bed particles (m)	$MAE$	Mean average error
$y_t$	tailwater depth (m)	$RMSE$	Root mean square error
$\phi$	flip-bucket angle (degree)	$\delta$	average absolute deviation.
$\Pi$	Dimensionless parameter		

$R$	$MAE$	$RMSE$	$\delta$
$R = \sqrt{1 - \frac{\sum_{i=1}^N (o_i - t_i)^2}{\sum_{i=1}^N (o_i - \bar{o}_i)^2}}$	$MAE = \frac{1}{N} \sum_{i=1}^N  o_i - t_i  * 100$	$RMSE = \sqrt{\frac{\sum_{i=1}^N (o_i - t_i)^2}{N}}$	$\delta = \frac{\sum  o_i - t_i }{\sum o_i} * 100$

**Conclusion**

Evaluation of the obtained results by the empirical relations presented by other researchers indicated that the relations could not appropriately predict the scour hole dimensions. Therefore, relations that provide better results are essential. For this reason, exponential and dimensionless relations were presented using 139 collected data series by applying SPSS19 software and  $\Pi$  Buckingham dimensional analysis and modifications to the coefficients and exponents by trial and error. Based on statistical analysis, the proposed relations provide appropriate results. Based on the evaluation of the proposed relations, the followings are concluded:

- Proposed relations showed a direct relation with the variables such as flow discharge per unit width, falling height, tailwater depth and angle of the flip-bucket so that by increasing each of these variables, scour depth will also increase.
- The exponent of  $d_{50}$  variable is negative for both relations, i.e. by increasing the average bed particle diameter, scour depth is decreased.
- The variables exponents in both relations are so close so that the results lie in the same numerical range.
- Interpretation of the confidence range determined in Figure 7 shows that a small number of calculated depths are outside the desired range. Therefore, high ratings are assigned to the proposed relations.
- A higher rating is assigned to the exponential relation in comparison to the dimensionless relation, but data distribution, correlation coefficients, computational error and the standard deviation of the both relations are almost the same.

**Appendix**

Statistical variables to evaluate the relations

To evaluate the accuracy of the proposed relations, the following statistical variables were used:

in which  $t_i$  denotes the target values of equilibrium scour depth ( $m$ ), while  $o_i$  and  $\bar{o}_i$  denote the observed and averaged observed values of equilibrium scour depth ( $m$ ), respectively, and  $N$  is the number of data points (Azamathullah & Zakaria 2011).

**REFERENCES**

Azamathulla, H. Md., Zakaria, N. A. 2011. Prediction of scour below submerged pipeline crossing a river using ANN. *Journal of Water Science & Technology* 63(10): 2225-2230.

Azamathullah, H. Md., Deo, M.C. and Deolalikar, P. B. 2005. Neural networks for estimation of scour downstream of ski-jump bucket. *ASCE, Journal of Hydraulic Engineering* 131 (10): 898- 908.

D’Agostino, V. and Ferro, V. 2004. Scour on Alluvial Bed Downstream of Grade-Control structures. *Journal of Hydraulic Engineering* 130 (1): 24-37.

Final report of the studies on the hydraulic model of the flood discharge system of Dariyan reservoir dam, Water Research Institute (affiliated with Power Ministry), Tehran, July 2013.

Final report of the studies on the hydraulic model of the flood discharge system of Azad reservoir dam, Water Research Institute (affiliated with Power Ministry), Tehran, September 2008.

Final report of the studies on the hydraulic model of the flood discharge system of Chereh reservoir dam, Water Research Institute (affiliated with Power Ministry), Tehran, Dec. 2008.

Ghodsian, M., Mehraein, M. and Ranjbar, H.R. 2012. Local scour due to free fall jets in non-uniform sediment. *Journal of Scientia Iranica A* 19(6): 1437-1444.

Guide to Local Scour calculation methods, No.549 Publication of the Technical Executive Office, May 2011.

Hoffmans, G. J. C. 1998. Jet scour in equilibrium phase. *Journal of Hydraulic Engineering, ASCE* 124 (4): 430-437.

Kuroiwa Zevallos, J. M and Minaya Espinoza, Elsa V. 2005. Scour in non-cohesive soil due to the impact of jet spillway out of ski jump. *Journal of Hydraulic Engineering, ASCE* 111(4): 1-11.

- Mason, P.J. and Arumugam, K. 1985. Free jet scour below dams and flip bucket. *Journal of Hydraulic Engineering*, ASCE 111 (2): 220-235.
- Robert. W. Fox Alant. Mc Donald, Philip J. Pritchard/ Introduction to Fluid Mechanics, Jhon Wiley and Sons, Inc., 2004.
- United states burean of reclamation, Design of small dams, third edition 1987.
- Veronese, A. 1937. Erosioni di fondo a valle di uno scarico. *Annal. Lavori Pubbl* 75(9): 717-726 (in Italian).
- Yen, C.L. 1987. Discussion on 'Free jet scour below dams and flip buckets' by Peter J. Mason and Kanapathypilly Arumugam. *Journal of Hydraulic Engineering*, ASCE113(9): 1200-1202.

\*\*\*\*\*